

IF YOU'VE EVER ASKED YOURSELF "WHY?" about something related to structural steel design or construction, *Modern Steel Construction*'s monthly Steel Interchange column is for you! Send us a question or comment to mod@aic.org.

Changes to Delegated Connection Designs

I have a contract in which the connections have been specified as Option 3 connections per the 2010 AISC *Code of Standard Practice*. That is, the connections are to be designed by our engineer with sealed calculations provided as the substantiating connection information. Shears and moments (for moment connections) were provided in the design drawings.

We submitted representative samples of the substantiating connection information, which showed the types of connections (shear tabs and bolted flange-plate moment connections) and the calculations we planned to submit to justify our connections. The final substantiating connection information was consistent with the representative samples, and we submitted it to the Structural Engineer of Record (SER) with the shop and erection drawings as a part of the approval process as given in Section 4.

The returned shop drawings were marked up to require one additional row of bolts in each shear connection and two additional bolts in each moment connection flange plate, even though the calculations we submitted demonstrate compliance with the requirements on the design drawings and in the AISC *Specification*, and recommendations in the AISC *Manual*.

When we asked why the additional bolts were necessary,

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Bolting Cost Comparison Article

There was a *Modern Steel Construction* article that discussed the relative costs of snug tight, pretensioned and slip-critical connections. In which issue of *MSC* did it appear?



www.modernsteel.com/backissues

Martin Anderson

Maximum Fillet Weld Size

AISC *Specification* Section J2.2b has a requirement that the maximum size of a fillet weld on material $\frac{1}{4}$ in. or more in thickness shall be $\frac{1}{16}$ in. less than the material thickness. Would this requirement apply to the case of a column-to-base-plate connection?

